

# CEE 577: Surface Water Quality Modeling

Lecture #10  
(Rivers & Streams, cont)  
Chapra, L14 (cont.)

# Longitudinal Dispersion

- From Fischer et al., 1979

$$E = 0.011 \frac{U^2 B^2}{HU^*}$$

$m^2s^{-1}$  →

→  $m/s$

→ Width (m)

→ Mean depth (m)

Where the Shear Velocity is:

$$U^* = \sqrt{gHS}$$

# Lateral Mixing

- Lateral or transverse dispersion coefficient for a stream:

$$E_{lat} = 0.6HU^*$$

Mean depth

Shear velocity

- Length required for complete mixing:

Side discharge:

$$L_m = 0.40U \frac{B^2}{E_{lat}}$$

Center discharge:

$$L_m = 0.10U \frac{B^2}{E_{lat}}$$

Width

# General Stream Geometry

- Chapra's nomenclature for discharge coefficients

- Velocity

$$U = aQ^b$$

- Depth

$$H = \alpha Q^\beta$$

- Width

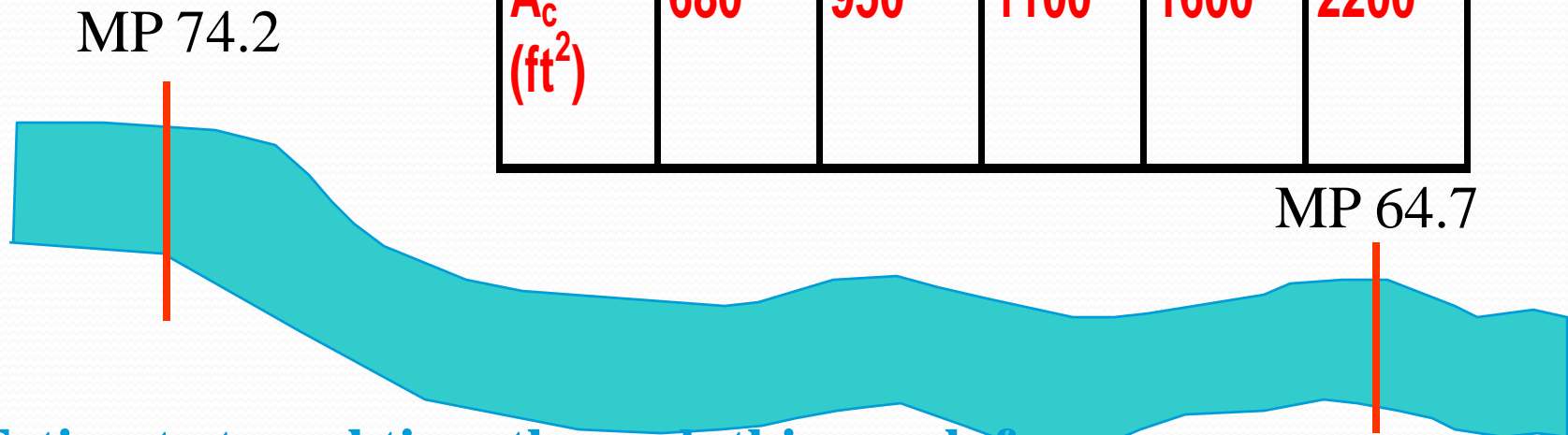
$$B = cQ^f$$

Where:  $b + \beta + f = 1$       Because  $Q = UHB$

# Sample Problem

The Black River, NY between MP 74.2 and MP 64.7 is to be characterized as a constant flow - constant area reach. Assume the following cross-sectional area ( $A_c$ ) were measured for the given flows:

<b>Q (cfs)</b>	<b>500</b>	<b>750</b>	<b>1300</b>	<b>2200</b>	<b>3400</b>
<b><math>A_c</math> (ft<sup>2</sup>)</b>	<b>680</b>	<b>950</b>	<b>1100</b>	<b>1600</b>	<b>2200</b>

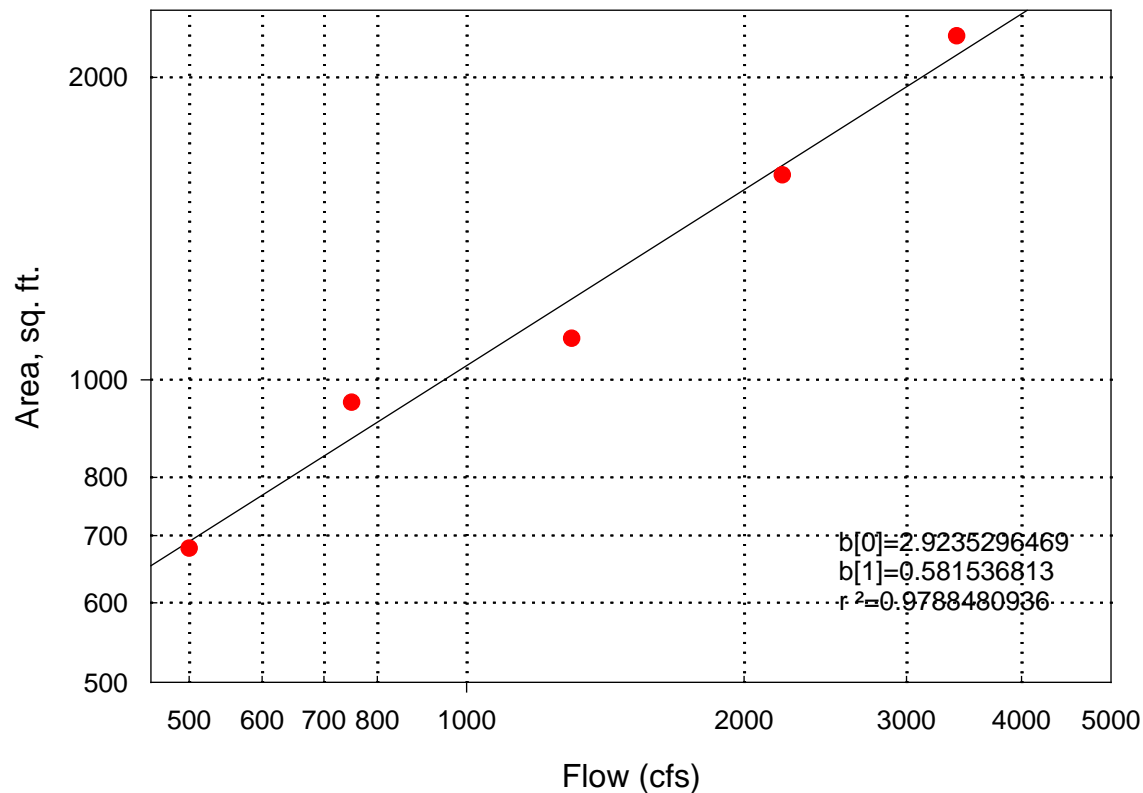


**Estimate travel time through this reach for flows of 600 and 3000 cfs**

$$H = \alpha Q^\beta$$

$$B = cQ^f$$

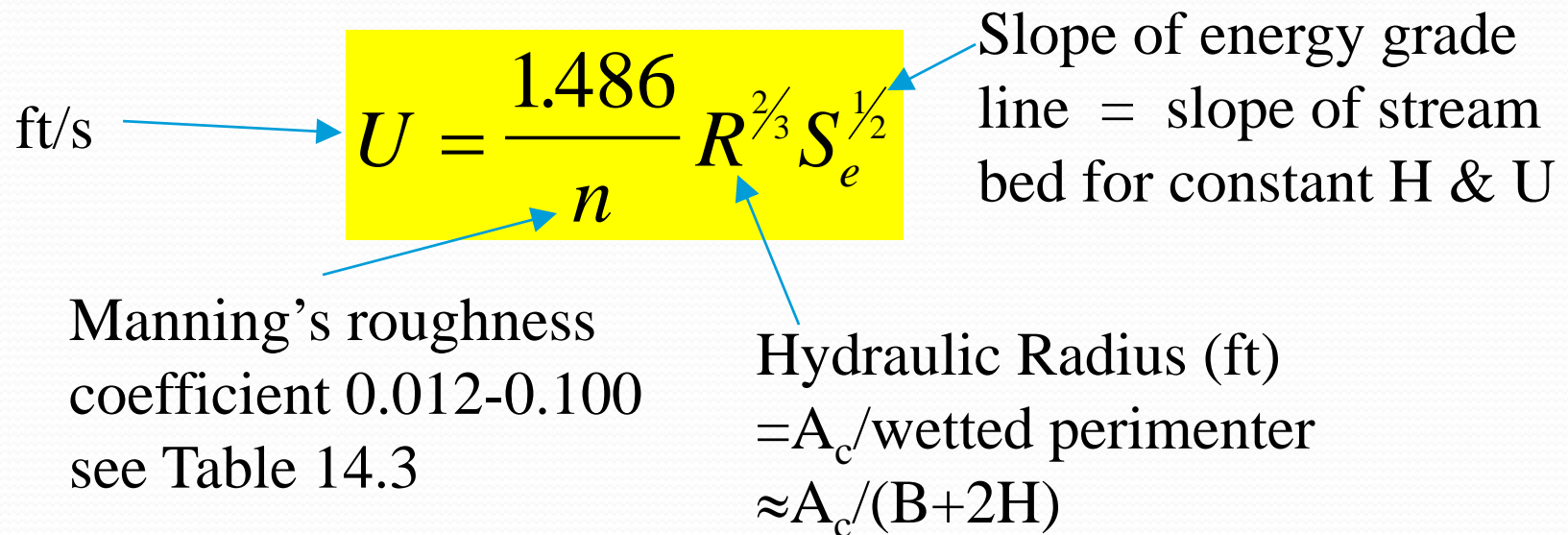
$$A = BH = \alpha c Q^{(\beta+f)}$$



$$A = 18.3Q^{0.589}$$

# Manning Equation

- Derived from the momentum balance
  - relates velocity to channel characteristics including slope



The diagram shows the Manning Equation  $U = \frac{1.486}{n} R^{2/3} S_e^{1/2}$  highlighted in a yellow box. Annotations include: 'ft/s' pointing to  $U$ ; 'Manning's roughness coefficient 0.012-0.100 see Table 14.3' pointing to  $n$ ; 'Hydraulic Radius (ft) =  $A_c$ /wetted perimeter  $\approx A_c/(B+2H)$ ' pointing to  $R$ ; and 'Slope of energy grade line = slope of stream bed for constant H & U' pointing to  $S_e$ .

$$U = \frac{1.486}{n} R^{2/3} S_e^{1/2}$$

ft/s

Manning's roughness coefficient 0.012-0.100 see Table 14.3

Hydraulic Radius (ft)  
=  $A_c$ /wetted perimeter  
 $\approx A_c/(B+2H)$

Slope of energy grade line = slope of stream bed for constant H & U

# Manning Equation adapted to a Trapezoidal section

- Area, perimeter and hydraulic radius can all be expressed as a function of depth
- substitute these into the Manning Equation and calculate “y” from known “Q”

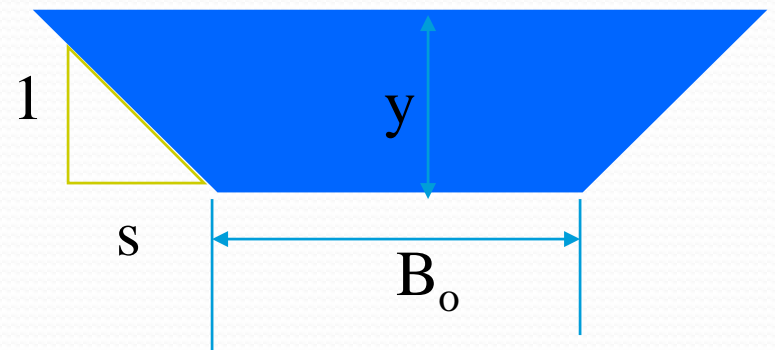
$$Q = \frac{1.486}{n} A_c R^{2/3} S_e^{1/2}$$

$$Q = \frac{1.486}{n} \frac{[(B_o + sy)y]^{5/3}}{(B_o + 2y\sqrt{s^2 + 1})^{2/3}} S_e^{1/2}$$

$$A_c = (B_o + sy)y$$

$$P = B_o + 2y\sqrt{s^2 + 1}$$

$$R = \frac{A_c}{P} = \frac{(B_o + sy)y}{B_o + 2y\sqrt{s^2 + 1}}$$

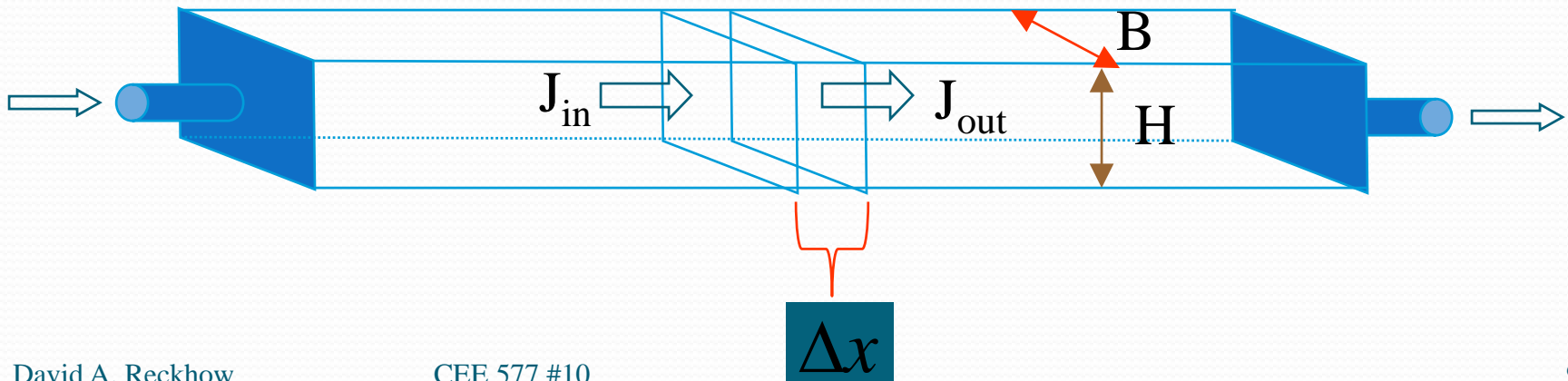




# Distributed Systems

- Lecture #9 in Chapra's book
  - systems that have spatial resolution
- Ideal Reactors

$$\Delta V \frac{\partial c}{\partial t} = J_{in} A_c - J_{out} A_c \pm reaction$$



# Plug-Flow Reactors (PRF)

$$\Delta V \frac{\partial c}{\partial t} = J_{in} A_c - J_{out} A_c \pm \text{reaction}$$

$$\Delta V \frac{\partial c}{\partial t} = U c A_c - U \left( c + \frac{\partial c}{\partial x} \Delta x \right) A_c - k \Delta V \bar{c}$$

Combining and taking the limit as  $\Delta x \rightarrow 0$

$$\frac{\partial c}{\partial t} = -U \frac{\partial c}{\partial x} - kc$$

Which at steady state is:

$$0 = -U \frac{\partial c}{\partial x} - kc$$

And for  $c=c_0$  at  $x=0$ :

$$c = c_0 e^{-kx/u}$$

# Plug Flow vs CSTR

- First order reactions

$$c = c_o e^{-kx/u}$$

$$c = c_o \frac{Q}{Q + kV}$$

Which is more efficient?

- Mixed Flow: intermediate
  - read section 9.1.3

# Mixed Flow

- Consider mixing in the longitudinal direction

$$\Delta V \frac{\partial c}{\partial t} = J_{in} A_c - J_{out} A_c \pm \text{reaction}$$

$$\Delta V \frac{\partial c}{\partial t} = \left[ U c - E \frac{\partial c}{\partial x} \right] A_c - \left[ U \left( c + \frac{\partial c}{\partial x} \Delta x \right) - E \left( \frac{\partial c}{\partial x} + \frac{\partial}{\partial x} \left( \frac{\partial c}{\partial x} \right) \Delta x \right) \right] A_c - k \Delta V c$$

Combining and taking the limit as  $\Delta x \rightarrow 0$

$$\frac{\partial c}{\partial t} = -U \frac{\partial c}{\partial x} + E \frac{\partial^2 c}{\partial x^2} - kc$$

Which at steady state is:

$$0 = -U \frac{\partial c}{\partial x} + E \frac{\partial^2 c}{\partial x^2} - kc$$

# Mixed Flow

$$\Delta V \frac{\partial c}{\partial t} = J_{in} A_c - J_{out} A_c \pm reaction$$

$$\Delta V \frac{\partial c}{\partial t} = \left[ U c - E \frac{\partial c}{\partial x} \right] A_c - \left[ U \left( c + \frac{\partial c}{\partial x} \Delta x \right) - E \left( \frac{\partial c}{\partial x} + \frac{\partial}{\partial x} \left( \frac{\partial c}{\partial x} \right) \Delta x \right) \right] A_c - k \Delta V \bar{c}$$

Consider mixing in the longitudinal direction

- Peclet Number

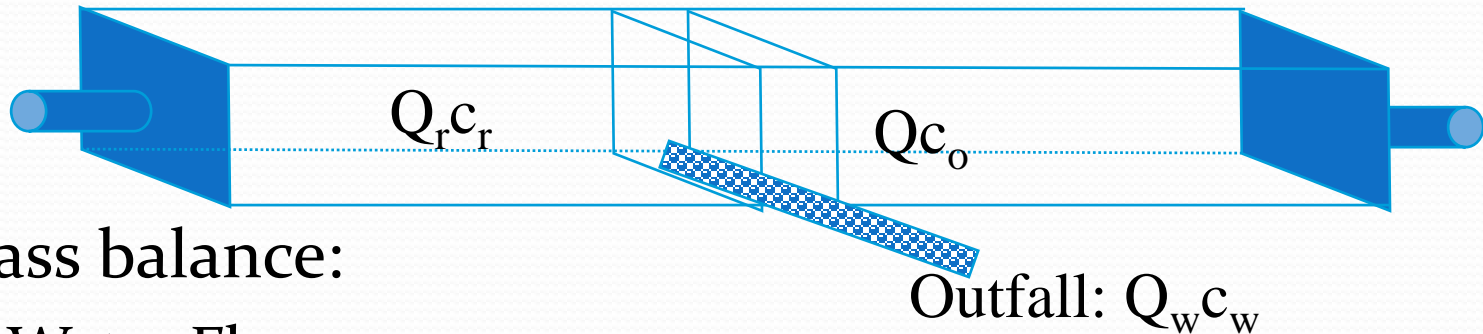
$$P_e = \frac{LU}{E} = \frac{\text{rate of advective transport}}{\text{rate of dispersive transport}}$$

$P_e > 10$ , PFR-like

$P_e < 0.1$ , CSTR-like

# Application of PRF to streams

- Point sources



- Mass balance:
  - Water Flow
  - Concentration

$$Q = Q_w + Q_r$$

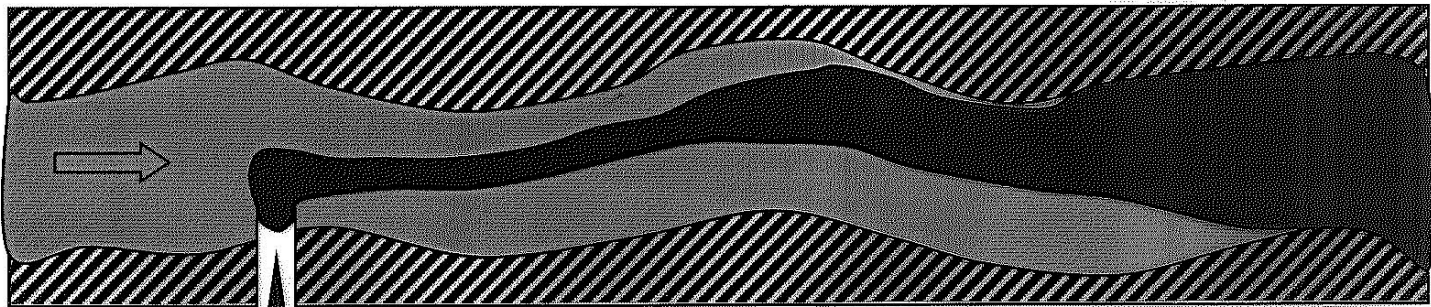
$$c_o = \frac{Q_w c_w + Q_r c_r}{Q_w + Q_r}$$

# Assumptions

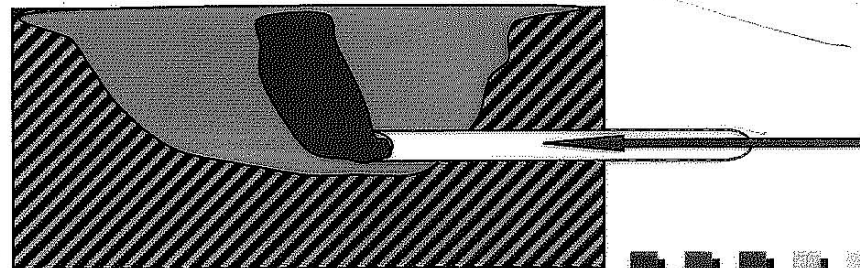
## ■ The ideal reactor is:

- Completely mixed in cross-section
- Without longitudinal (downstream) mixing
- Steady State

Top view



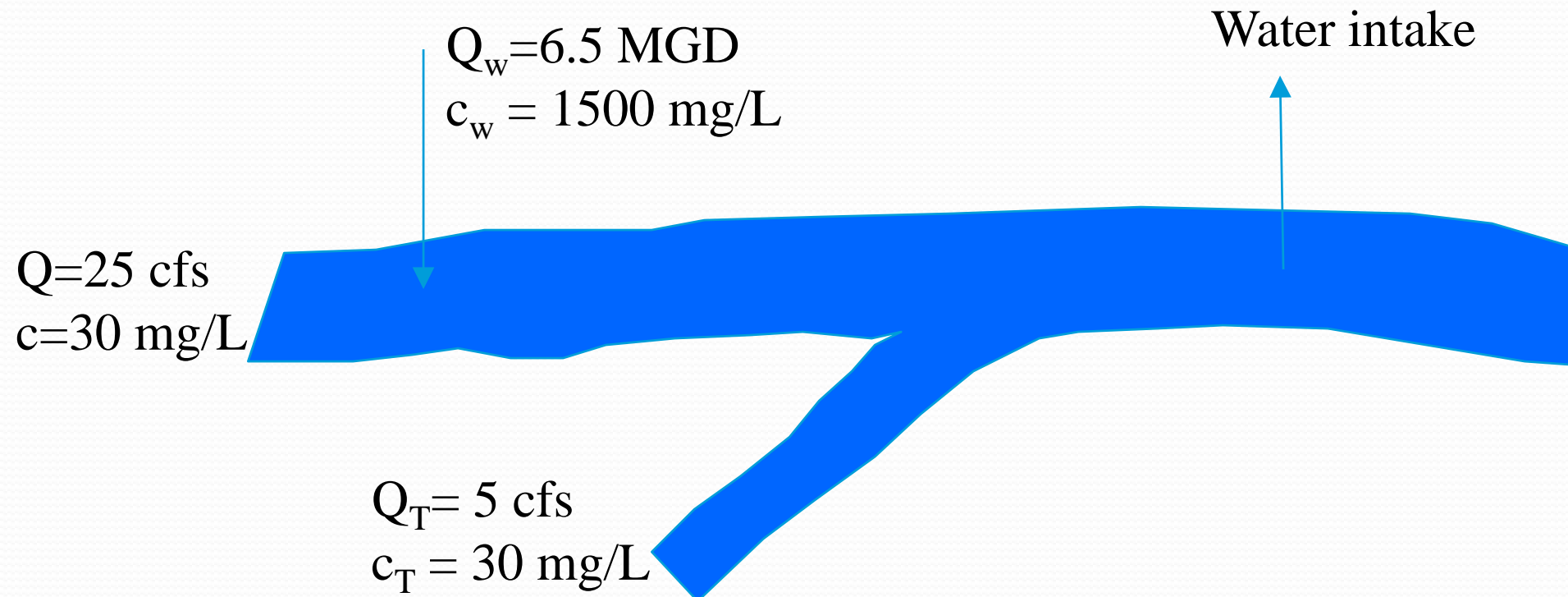
Cross-section



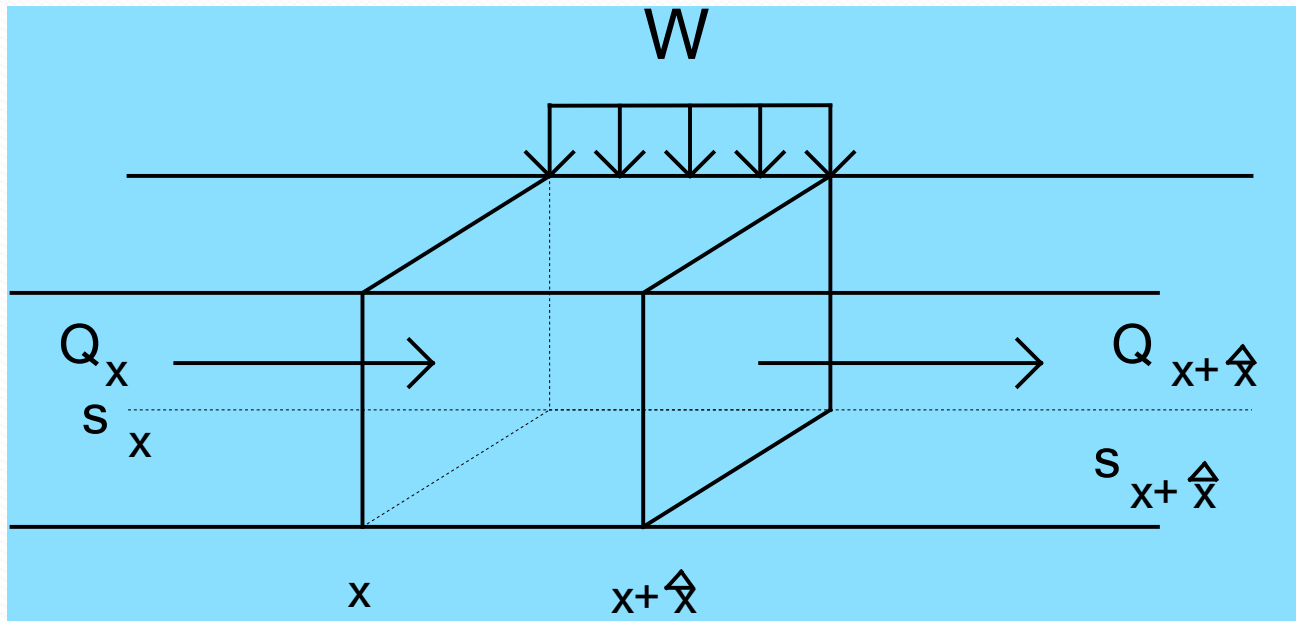
# Chloride Problem

1.55 cfs/MGD

- Determine the required industrial reduction in chlorides to maintain a desired chloride concentration of 250 mg/L at the intake







- To next lecture