

CEE 370

Environmental Engineering Principles



Lecture #11

Ecosystems I: Water & Element Cycling, Ecological Principles

Reading: Mihelcic & Zimmerman, Chapter 4

Davis & Masten, Chapter 4

Monday's local paper

Daily Hampshire Gazette



Monday September 30, 2019

gazettenet.com

Established 1786

HOLYOKE

6 displaced in severe house fire

Mayor calls dysfunctional hydrant at scene 'extremely concerning'

By GRETA JOCHEM
Staff Writer

HOLYOKE — On Sunday, all that remained of the home at 68 Fairfield Avenue was a mound of burned wood, bricks and rubble.

A fire destroyed the two-story home Saturday night displacing its six residents, including an infant. Issues with nearby fire hydrants prompted Mayor Alex Morse to call for an audit of the city's hydrants, saying the problem was "extremely concerning." Holyoke Fire Department Captain Kevin Cavagnac, meanwhile, said the hydrant problem "didn't hamper" their firefighting efforts.

The whole house is lost, said Cavagnac, and he noted that the cause of

the fire was an improperly disposed cigarette. Everything inside it is also gone, said Janat Langevin, a resident of the house. "Everything we owned. Every single thing we've owned," she said while sitting in her neighbor's yard on Sunday afternoon. "I didn't even get my pocket-book."

The blaze broke out at around 6 p.m. on the home's front porch, officials said, and flames would soon engulf the entire home, located near Northampton Street in the Fairfield Avenue Historic District. Four adults were in the home at the time, and the fifth was on a walk with the baby, according to Cavagnac.

Langevin said she was in the kitchen when her husband told her they needed to get out of the house, and they went outside. "It just went up like something you'd see in the movie," she said. "I've never seen anything like that."

Around the same time, Marco

Crescentini, who lives directly next door, was watching a movie with his son who noticed the fire next door. Crescentini went to his front lawn and saw his neighbor's front porch ablaze.

"Then we heard some yelling," he said. He saw two adults on the second-floor porch shouting for help. The two people were cut off from the stairs by smoke, according to Cavagnac.

Crescentini ran into the garage and got a ladder and propped it against the house so they could escape. "It was definitely adrenaline," he said. "It was really nothing heroic."

He added, "It was sad to see it go down. Their personal loss is immense."

Firefighters battled the blaze for hours as sections of the home collapsed. While the fire burned, it was



STAFF PHOTO/DAVID CROWLEY

and he noted that the cause of

CEE 370-L #11

HOLYOKE FIRE #8

A fire destroyed a home at 68 Fairfield Ave. on Saturday night.

Daily Hampshire Gazette



Tuesday October 1, 2019

gazettenet.com

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HOLYOKE

with low water pressure

The problem, Holyoke Water Works general manager David Conti said, is that some pipes in the city are 100 years old. Inside those pipes, tuberculation — the buildup of mounds of rust — shrinks the diameter of the pipe, limiting water pressure.



on bond to address the problem of low water flow in neighborhoods, Morse said Monday. He said the department had assured every fire hydrant is operat

and the water department do another check available," Morse said.

as almost 2,000 fire hydrants said that they are regularly, as well as after every

that the Fire Department of areas with water restrictions and that nearby with sufficient water have been marked with

rare. There are only a few areas where this problem exists," Conti said. "Under

those circumstances, the Fire Department would have to go a significant distance — up to 100 feet away — to connect to a fire hydrant that has the capability of fighting a fire of that magnitude."

The problem, Conti said, is that some pipes in the city are 100 years old. Inside those pipes, tuberculation — the buildup of mounds of rust — shrinks the diameter of the pipe, limiting water pressure.

"Every city, every community is

going to have areas where they have limitations," Conti said.

Conti said the problem areas were not addressed in previous years because the city was paying off a bond for several state-mandated capital projects.

"Now that these state projects have been paid off, these bonds have matured, immediately we went to developing a priority schedule to iden-

SEE PRESSURE A7

Pressure

FROM A1

tify these areas so the city could get on board and replace these water mains," Conti said.

The replacement projects are now in their design phase, and should be completed in two or three years, Conti said. The bond will go toward paying for the 22 projects identified as "high priority," but not 14 other projects that are considered ei-

ther "medium priority" or "low priority."

In the meantime, Conti said the water department will need to sit down with the Fire Department to make sure they remember which areas have low water pressure. Morse said that the city would put together a comprehensive map of those areas.

Dusty Christensen can be reached at dchristensen@gazettenet.com.

Follow-up on Tuesday



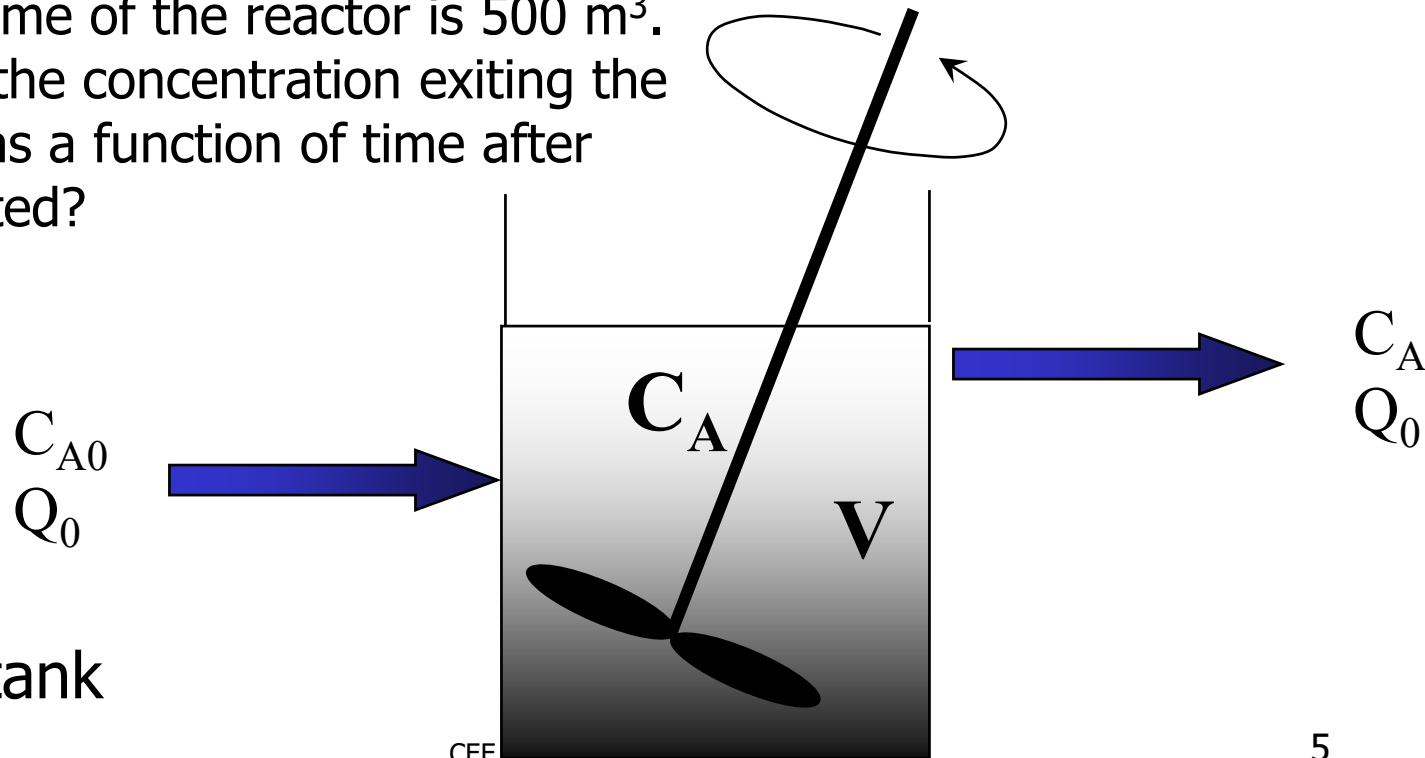
CMFR: non-SS

- Step loads are easier
- More complicated when reaction is occurring
- Simpler for case without reaction
 - Example 4-5 (pg 128-129)
- With reaction
 - Example 4-4 (pg 125-127)

Non-steady State CMFR

■ Problem:

- The CMFR is filled with clean water prior to being started. After start-up, a waste stream containing 100 mg/L of a conservative substance is added to the reactor at a flow rate of 50 m³/day. The volume of the reactor is 500 m³. What is the concentration exiting the reactor as a function of time after it is started?



Equalization tank



Non SS CMFR (cont.)

- So the general reactor equation reduces to:

$$\frac{dm_A}{dt} = C_{in} Q - C_{out} Q - r_A V$$

- And because we've got a conservative substance, $r_A=0$:

$$V \frac{dC_A}{dt} = C_{in} Q - C_{out} Q$$

$$\frac{dC_A}{dt} = \frac{Q}{V} (C_{in} - C_{out})$$

- Now let:

$$y = C_{out} - C_{in}$$



Non SS CMFR without reaction

- So that:

$$\frac{dy}{dt} = -\frac{Q}{V}y$$

- Rearranging and integrating,

$$\int_{y(0)}^{y(t)} \frac{dy}{y} = \int_0^t -\frac{Q}{V} dt$$

- Which yields,

$$\ln\left(\frac{y(t)}{y(0)}\right) = -\frac{Q}{V}t$$

- or
$$\frac{y(t)}{y(0)} = e^{-\left(\frac{Q}{V}\right)t}$$

Non SS CMFR w/o reaction (cont.)

- And substituting back in for y:

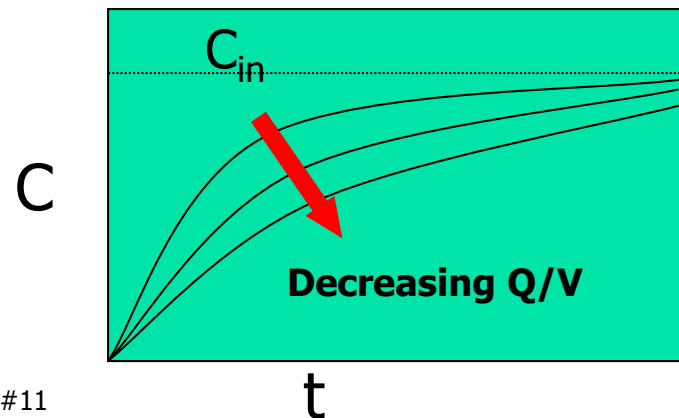
$$\frac{C - C_{in}}{C_o - C_{in}} = e^{-\left(\frac{Q}{V}\right)t}$$

- Since we're starting with clean water, $C_o = 0$

$$\frac{C - C_{in}}{-C_{in}} = e^{-\left(\frac{Q}{V}\right)t} \quad \text{and} \quad C - C_{in} = -C_{in} e^{-\left(\frac{Q}{V}\right)t}$$

- And finally,

$$C = C_{in} \left(1 - e^{-\left(\frac{Q}{V}\right)t} \right)$$



Now add a reaction term

- Returning to the general reactor equation:

$$\frac{dm_A}{dt} = C_{in} Q - C_{out} Q - r_A V$$

- And now we've got a 1st order reaction, $r_A = kC = kC_{out}$:

$$V \frac{dC}{dt} = C_{in} Q - C_{out} Q - kVC_{out}$$

$$\frac{dC}{dt} = \frac{Q}{V} (C_{in} - C_{out}) - kC_{out}$$

- This is difficult to solve, but there is a particular case with an easy solution: where $C_{in} = 0$
 - This is the case where there is a step decrease in the influent concentration to zero (M&Z, example 4.4)

Non SS CMFR; $C_{in}=0$

- So that:

$$\frac{dC}{dt} = -\frac{Q}{V}(C_{out}) - kC_{out}$$

- Rearranging, recognizing that in a CMFR, $C=C_{out}$, and integrating,

$$\frac{dC}{C} = -\left(\frac{Q}{V} + k\right) dt \qquad \int_{C(0)}^{C(t)} \frac{dC}{C} = -\int_0^t \left(\frac{Q}{V} + k\right) dt$$

- Which yields,

$$\ln\left(\frac{C(t)}{C(0)}\right) = -\left(\frac{Q}{V} + k\right)t$$

- or

$$\frac{C(t)}{C(0)} = e^{-\left(\left(\frac{Q}{V}\right) + k\right)t}$$

SS Comparison of PFR & CMFR

1st order reaction

■ CMFR

$$C_A = \frac{C_{A0}}{1 + \left(\frac{V}{Q}\right)k}$$

$$\frac{C_A}{C_{A0}} = \frac{1}{1 + \left(\frac{V}{Q}\right)k}$$

Example: $V=100\text{L}$, $Q=5.0\text{ L/s}$, $k=0.05\text{ s}^{-1}$

$$\frac{C_A}{C_{A0}} = \frac{1}{1 + \left(\frac{100}{5}\right)0.05}$$
$$= 0.50$$

■ PFR

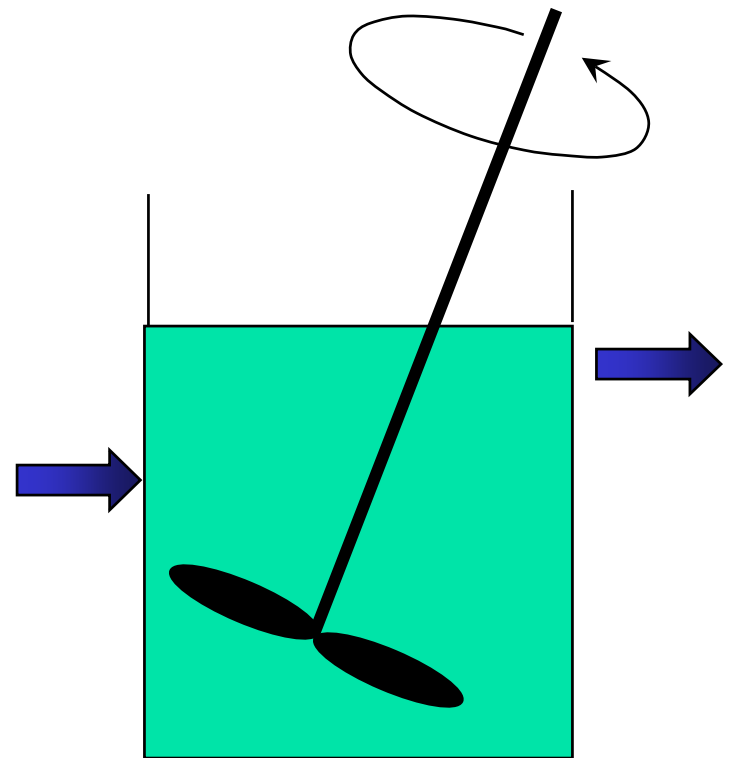
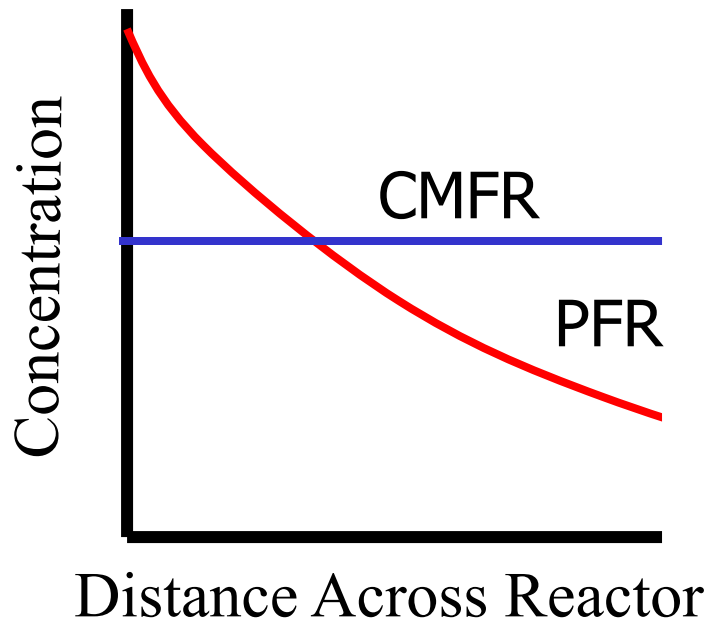
$$C_A = C_{A0} e^{-kV/Q}$$

$$\frac{C_A}{C_{A0}} = e^{-k\left(\frac{V}{Q}\right)}$$

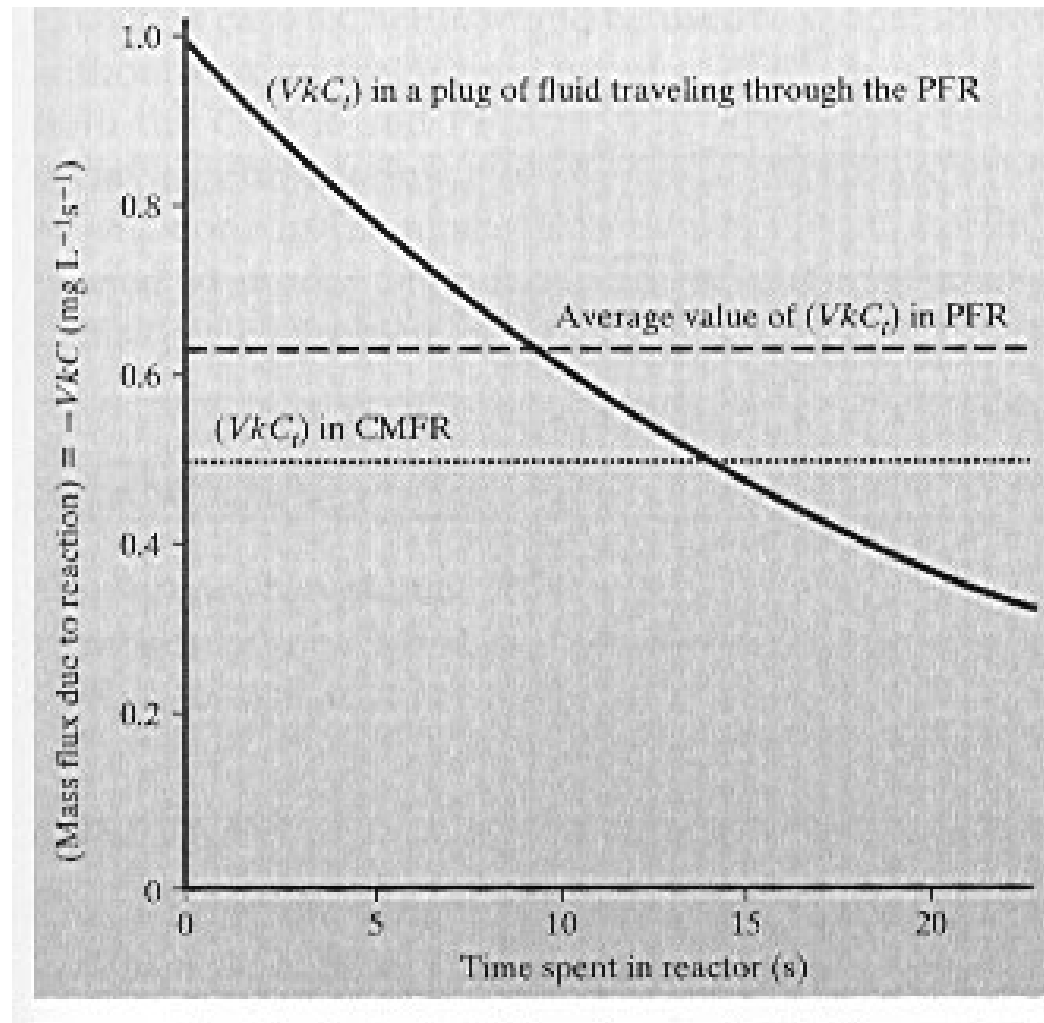
$$\frac{C_A}{C_{A0}} = e^{-0.05\left(\frac{100}{5}\right)}$$
$$= 0.37$$

- Conclusion:

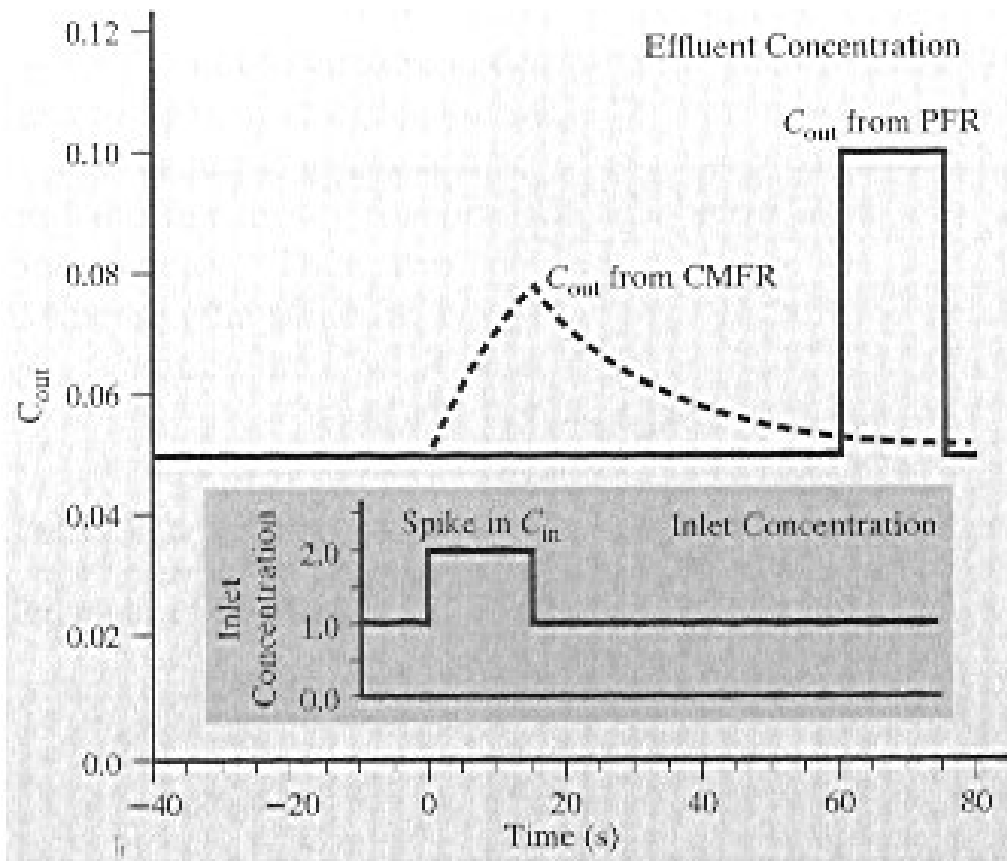
- PFR is more efficient for a 1st order reaction



- Rate of reaction of A is given by
 - VkC_A



Response to Inlet Spikes





Selection of CMFR or PFR

- PFR
 - Requires smaller size for 1st order process
- CMFR
 - Less impacted by spikes or toxic inputs



Comparison

- Davis & Masten, Table 4-1, pg 157

Comparison of Steady-state Performance for Decay Reactions of Different Order^a

Reaction Order	r	Equations for C_t		
		Ideal Batch	Ideal Plug Flow	Ideal CMFR
Zero ^b $t \leq C_0/k$ $t > C_0/k$	$-k$	$C_0 - kt$ 0	$C_0 - k\theta$	$C_0 - k\theta$
First	$-kC$	$C_0[\exp(-kt)]$	$C_0[\exp(-k\theta)]$	$\frac{C_0}{1 + k\theta}$
Second	$-2kC^2$	$\frac{C_0}{1 + 2ktC_0}$	$\frac{C_0}{1 + 2k\theta C_0}$	$\frac{(8k\theta C_0 + 1)^{1/2} - 1}{4k\theta}$

^a C_0 = initial concentration or influent concentration; C_t = final condition or effluent concentration.

^bTime conditions are for ideal batch reactor only.



Retention Time

$$\theta = \frac{V}{Q}$$

Table 4-3. Typical Retention Times in Unit Processes Used for Treating Drinking Water and Wastewater

Unit Operation	Used for	Approximate Retention Time
<i>Wastewater Treatment</i>		
Grit removal	Removal of large particles (grit)	30 min
Primary settling	Removal of large solids	≤ 1 h
Secondary Settling	Removal of smaller solids	≤ 2 h
Activated sludge	Removal of organic matter using microorganisms and oxygen	4–8 h
Anaerobic digester	Stabilization of organic matter in sludge in absence of oxygen	15–30 days
<i>Drinking-water Treatment</i>		
Rapid-mix tank	Blending of chemical coagulants with water prior to treatment	< 1 min
Flocculator	Gentle mixing to promote flocculation of small particles	30 min
Disinfection	Destruction of pathogens	< 15 min



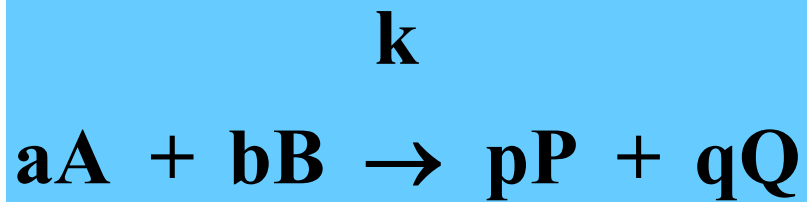
Analysis of Treatment Processes

- Basic Fluid Principles
 - Volumetric Flow Rate
 - Hydraulic Retention Time
- Conversion
- Mass Balances
- Reaction Kinetics and Reactor Design
 - Chemical Reaction Rates
 - Reactor Design
- Sedimentation Principles



Conversion or Efficiency

Stoichiometry



And the Conversion, X , is:

$$X = \frac{(C_{A0} - C_A)}{C_{A0}}$$

or

$$C_A = C_{A0}(1 - X)$$

Some use "efficiency" (η) to indicate the same concept

Conversion/efficiency (cont.)

$$(N_{B0} - N_B) = \frac{b}{a}(N_{A0} - N_A)$$

where,

N_{A0} = moles of A at $t = 0$, [moles]

N_A = moles of A at $t = t$, [moles]

N_{B0} = moles of B at $t = 0$, [moles]

N_B = moles of B at $t = t$, [moles]

If the volume of the reactor is assumed to remain constant, we can divide both sides of the expression by $C_{A0}V$. The expression then becomes,

$$\frac{(C_{B0} - C_B)}{C_{A0}} = \frac{b(C_{A0} - C_A)}{a C_{A0}} = \frac{b}{a}X$$



Conversion/efficiency (cont.)

This expression can then be solved for the concentration of B in terms of other known quantities:

$$C_B = C_{B0} - \frac{b}{a} C_{A0} X$$

Conversion/efficiency Example

The reactor shown in the Figure has an inflow of 750 L/hr. The concentration of A in the influent is 0.3 M and the concentration of B in the influent is 0.5 M. The conversion (of A) is 0.75. The reaction is:

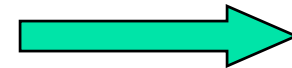
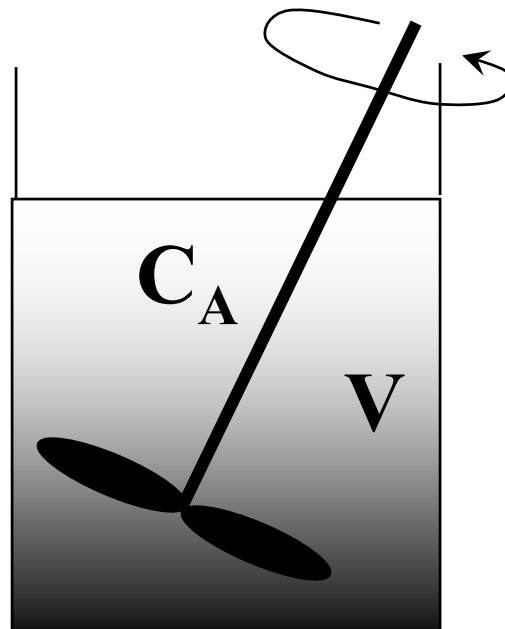
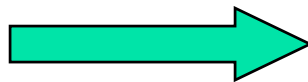


Find the conversion of B, X_B , and the effluent concentration of A and B.

$$C_{A_0} = 0.3M$$

$$C_{B_0} = 0.5M$$

$$Q = 750 \text{ L/hr}$$



$$C_A = ?$$

$$C_B = ?$$



Solution to Conversion Ex.

The first step in the solution is to determine the effluent concentration of A. This can be obtained as follows:

$$C_A = C_{A0}(1 - X) = 0.3 \text{ M} \times (1 - 0.75)$$

$$C_A = 0.075 \text{ M}$$

For each mole of A converted to product, two moles of B are converted to product. Since we know the initial concentration of B we can calculate its final concentration:

$$\text{Moles/ L of B converted} = 2 \times (0.3 \text{ M} - 0.075 \text{ M}) = 0.45 \text{ M}$$



Solution to Conversion Ex. (cont.)

$$C_B = 0.5 \text{ M} - 0.45 \text{ M} = 0.05 \text{ M}$$

Alternatively, using Eqn:

$$C_B = C_{B_0} - \frac{b}{a} C_{A_0} X = 0.5 \text{ M} - \left(\frac{2}{1} \right) (0.3 \text{ M} \times 0.75)$$

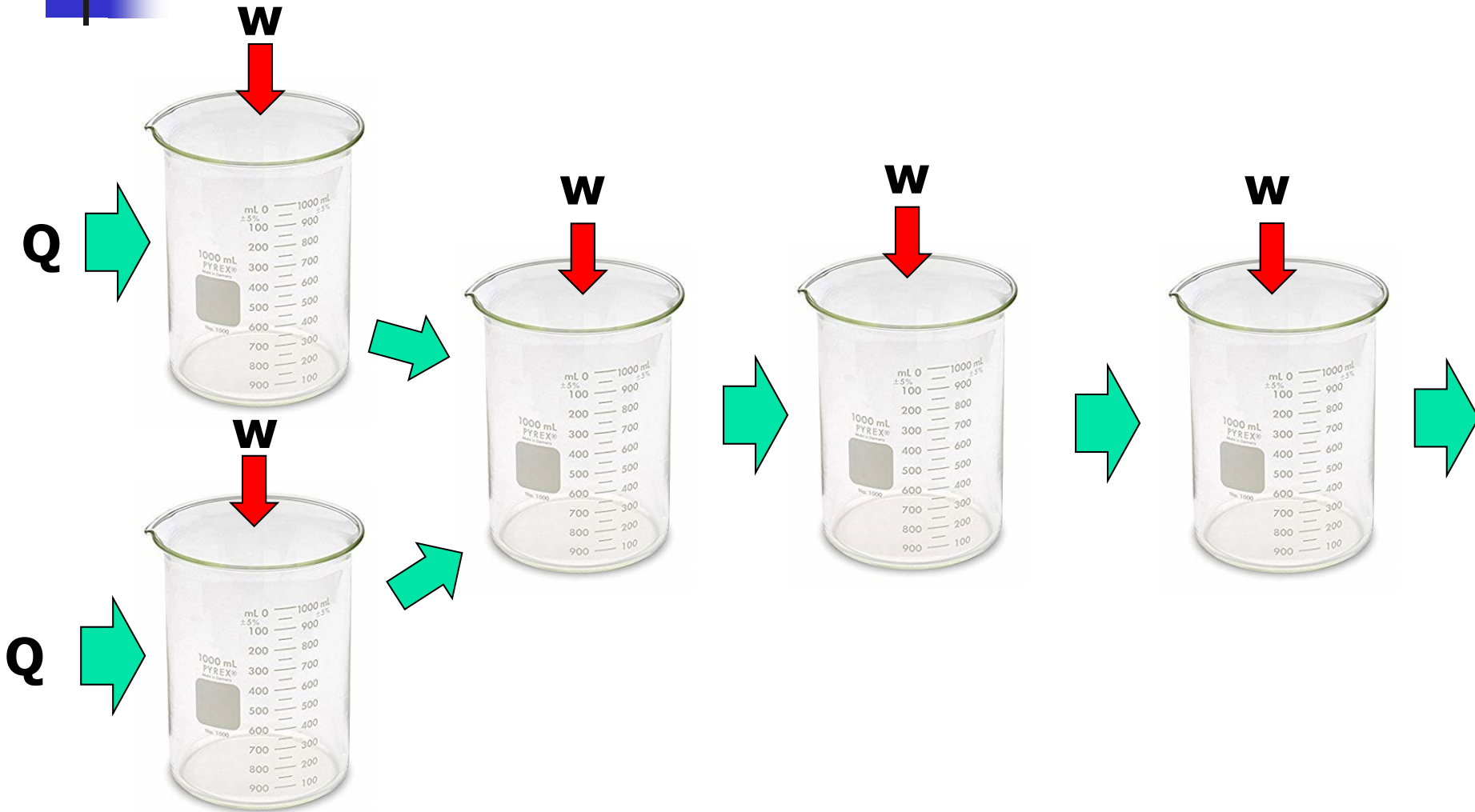
$$C_B = 0.05 \text{ M}$$

The conversion of B is then:

$$X_B = \frac{(C_{B_0} - C_B)}{(C_{B_0})} = \frac{0.5 \text{ M} - 0.05 \text{ M}}{0.5 \text{ M}}$$

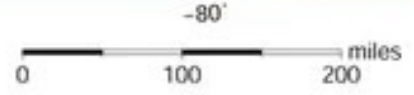
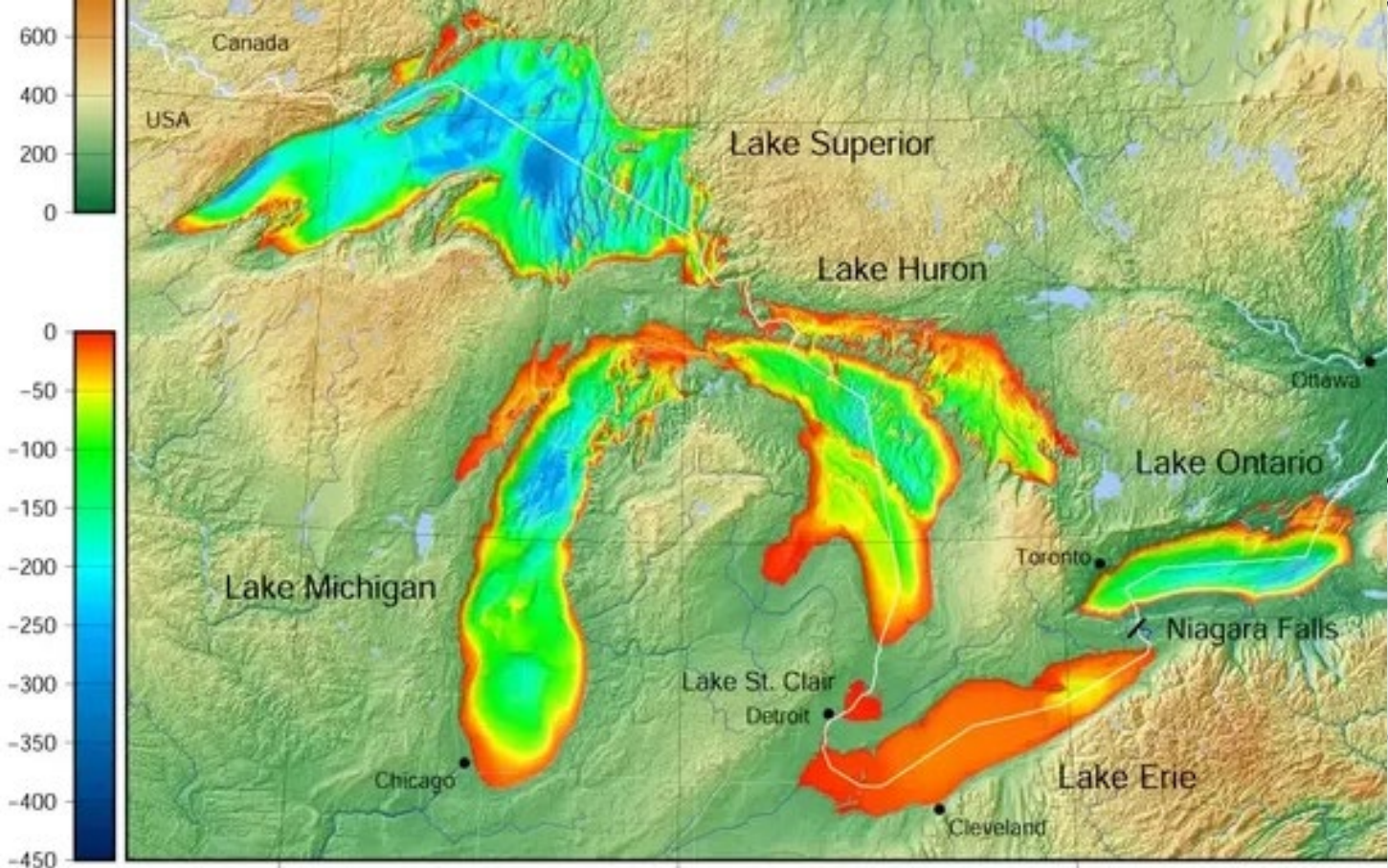
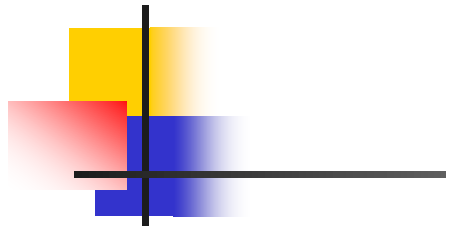
$$X_B = 0.9$$

Reactors in Series



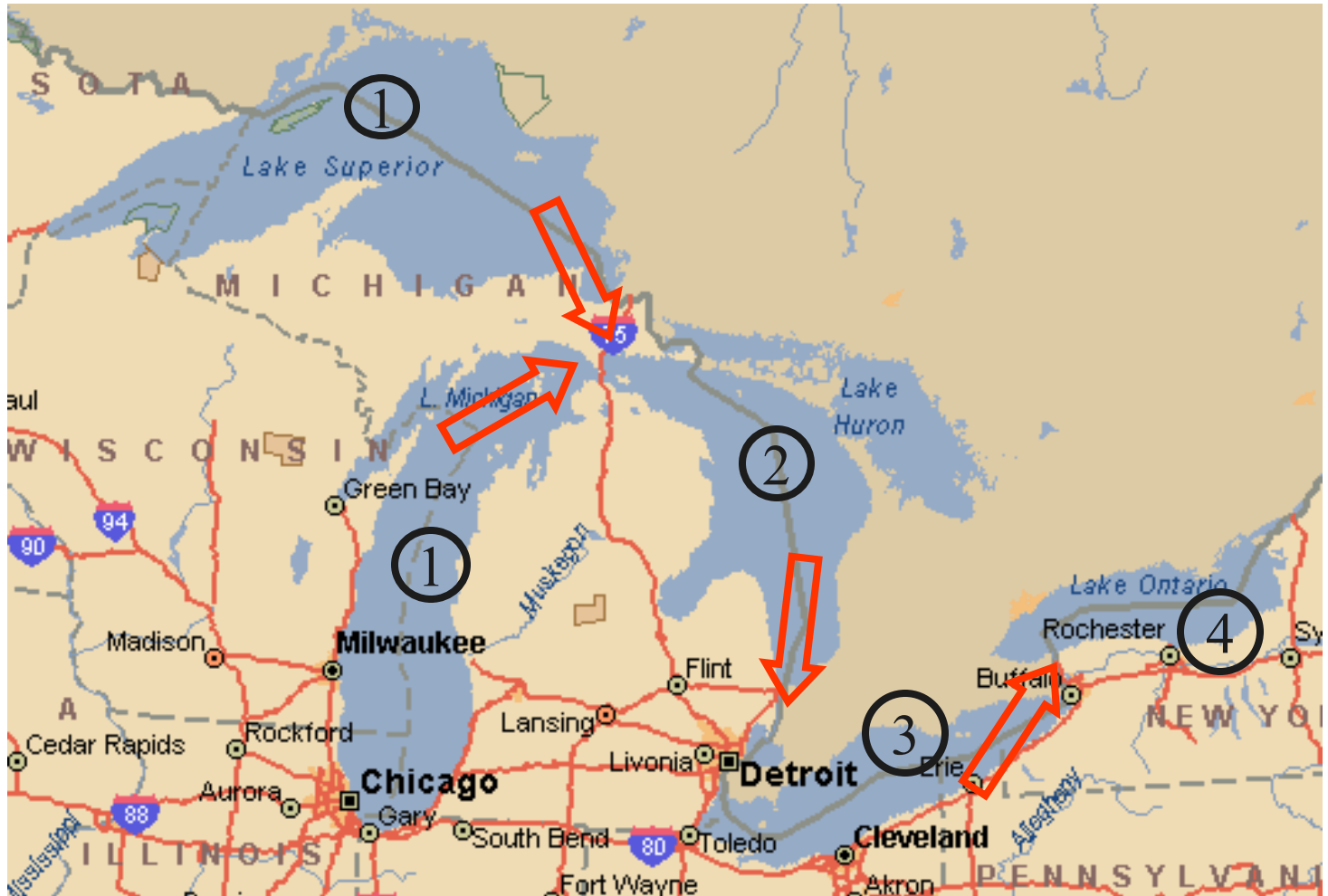
Reactors in Series



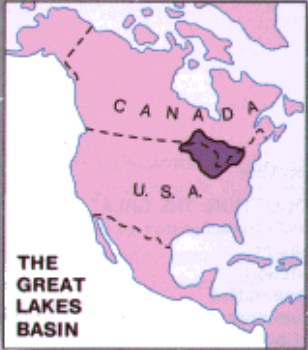


Lake	Volume (10^9 m^3)	Outflow ($10^9 \text{ m}^3 \text{ y}^{-1}$)
Superior	12,000	67
Michigan	4,900	36
Huron	3,500	161
Erie	468	182
Ontario	1,634	211

Example: ^{90}Sr fallout in Great Lakes



RELIEF, DRAINAGE AND URBAN AREAS



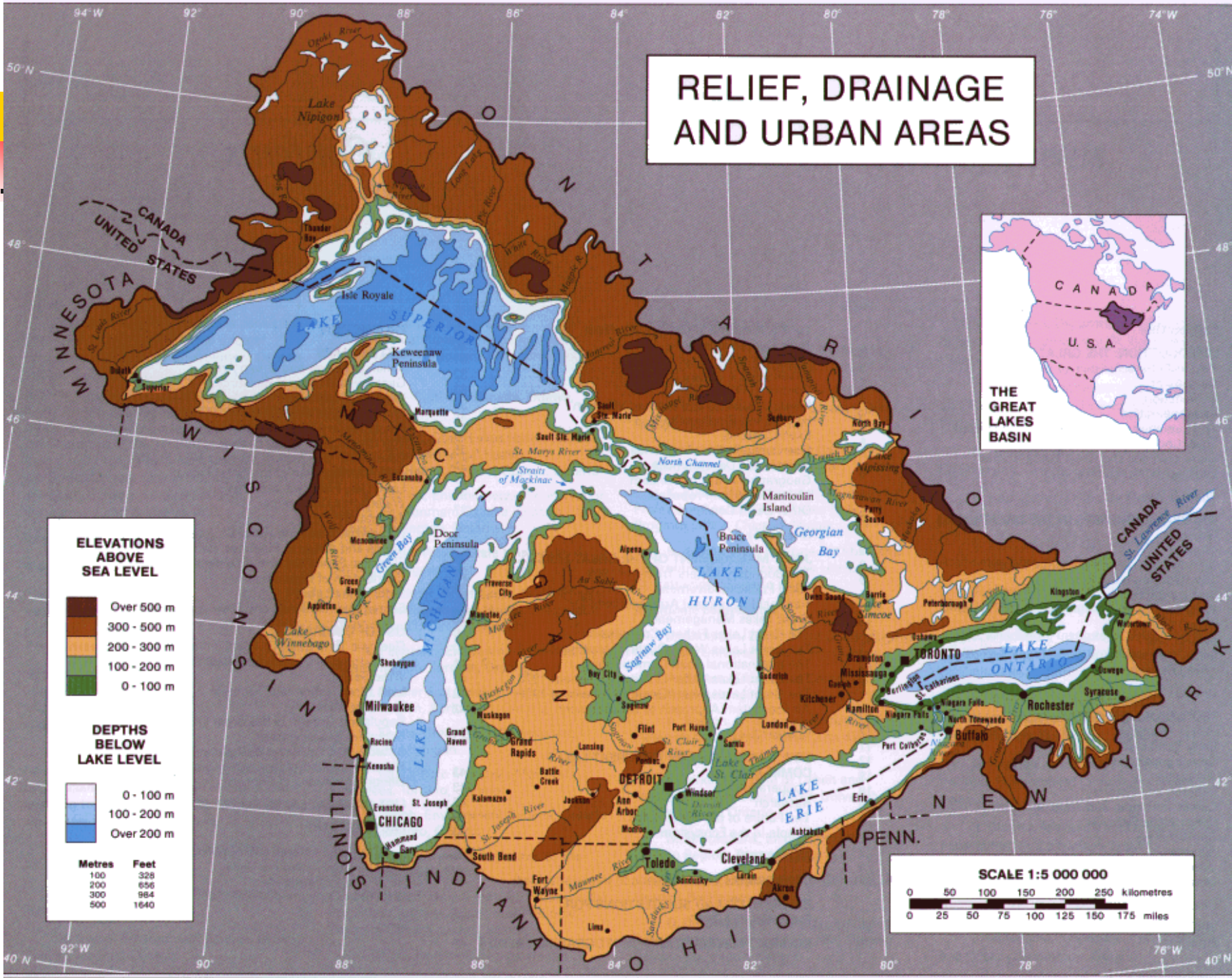
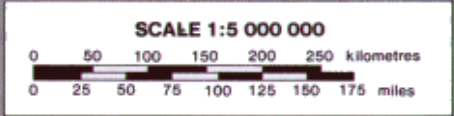
ELEVATIONS ABOVE SEA LEVEL

- Over 500 m
- 300 - 500 m
- 200 - 300 m
- 100 - 200 m
- 0 - 100 m

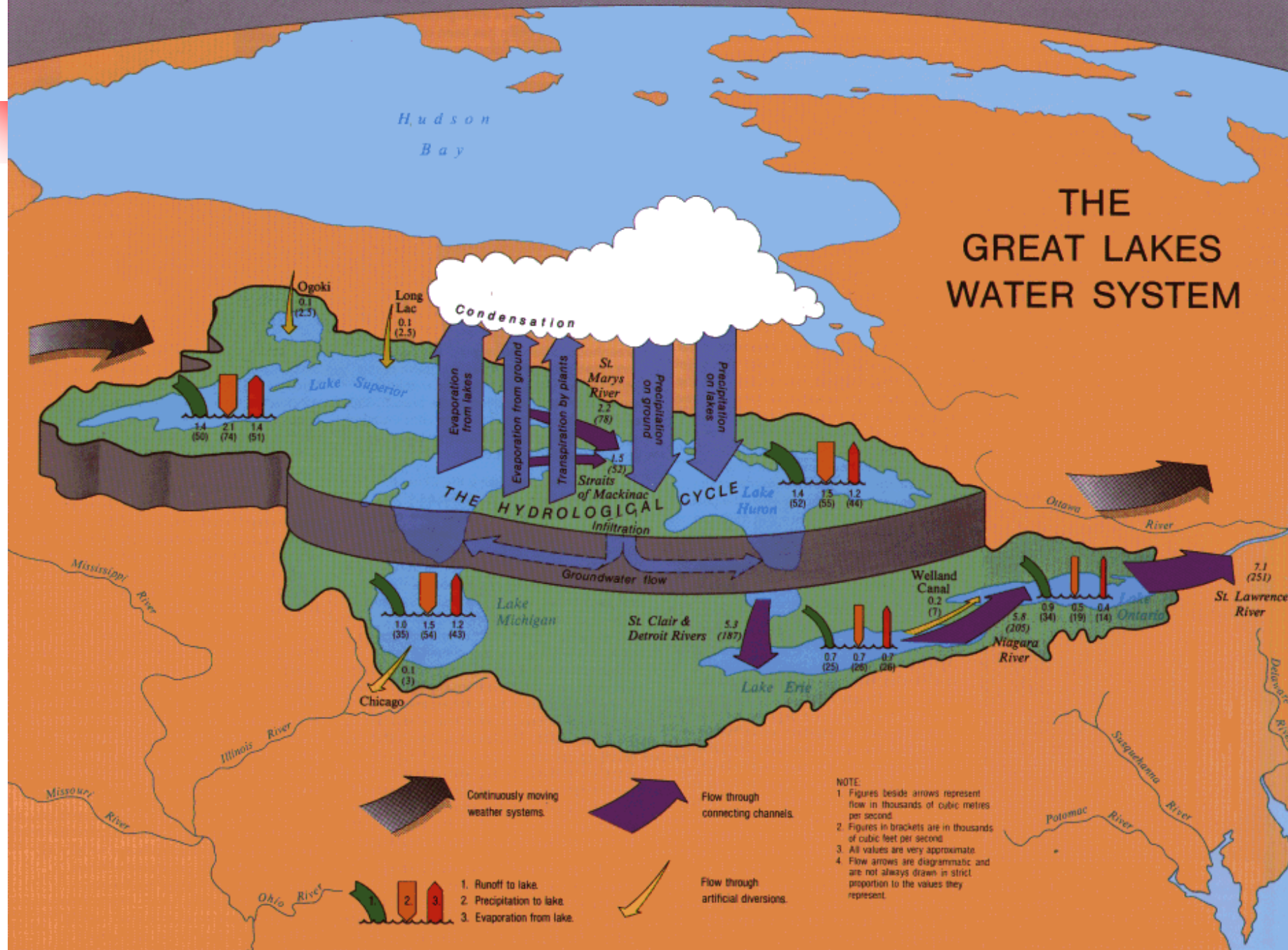
DEPTHS BELOW LAKE LEVEL

- 0 - 100 m
- 100 - 200 m
- Over 200 m

Metres	Feet
100	328
200	656
300	984
500	1640



THE GREAT LAKES WATER SYSTEM



Hudson Bay

Ogoki
0.1
(2.5)

Long Lac
0.1
(2.5)

14 (56) 21 (74) 14 (51)

St. Marys River
2.2
(78)

1.5
(52)

14 (52) 15 (55) 12 (44)

1.0 (35) 1.5 (54) 1.2 (43)

0.1
(3)

5.3
(187)

0.7 (25) 0.7 (26) 0.7 (26)

Welland Canal
0.2
(7)

5.8 (205)

0.9 (34) 0.5 (19) 0.4 (14)

7.1
(251)

Mississippi River

Illinois River

Missouri River

Ohio River

Potomac River

Susquehanna River

Delaware River

ECOREGIONS, WETLANDS AND DRAINAGE BASINS

MAJOR WETLANDS

There are numerous wetlands in northern Ontario and elsewhere that are too small to show individually at this scale.

NOTE:

Ecoregions are areas that exhibit broad ecological unity, based on such characteristics as climate, landforms, soils, vegetation, hydrology and wildlife.

CANADIAN ECOREGIONS

- 1 Lake St. Joseph Plains
- 2 Nipigon Plains
- 3 Thunder Bay Plains
- 4 Superior Highlands
- 5 Matagami
- 6 Chapleau Plains
- 7 Nipissing
- 8 Hurontario
- 9 Erie
- 10 Saint Laurent

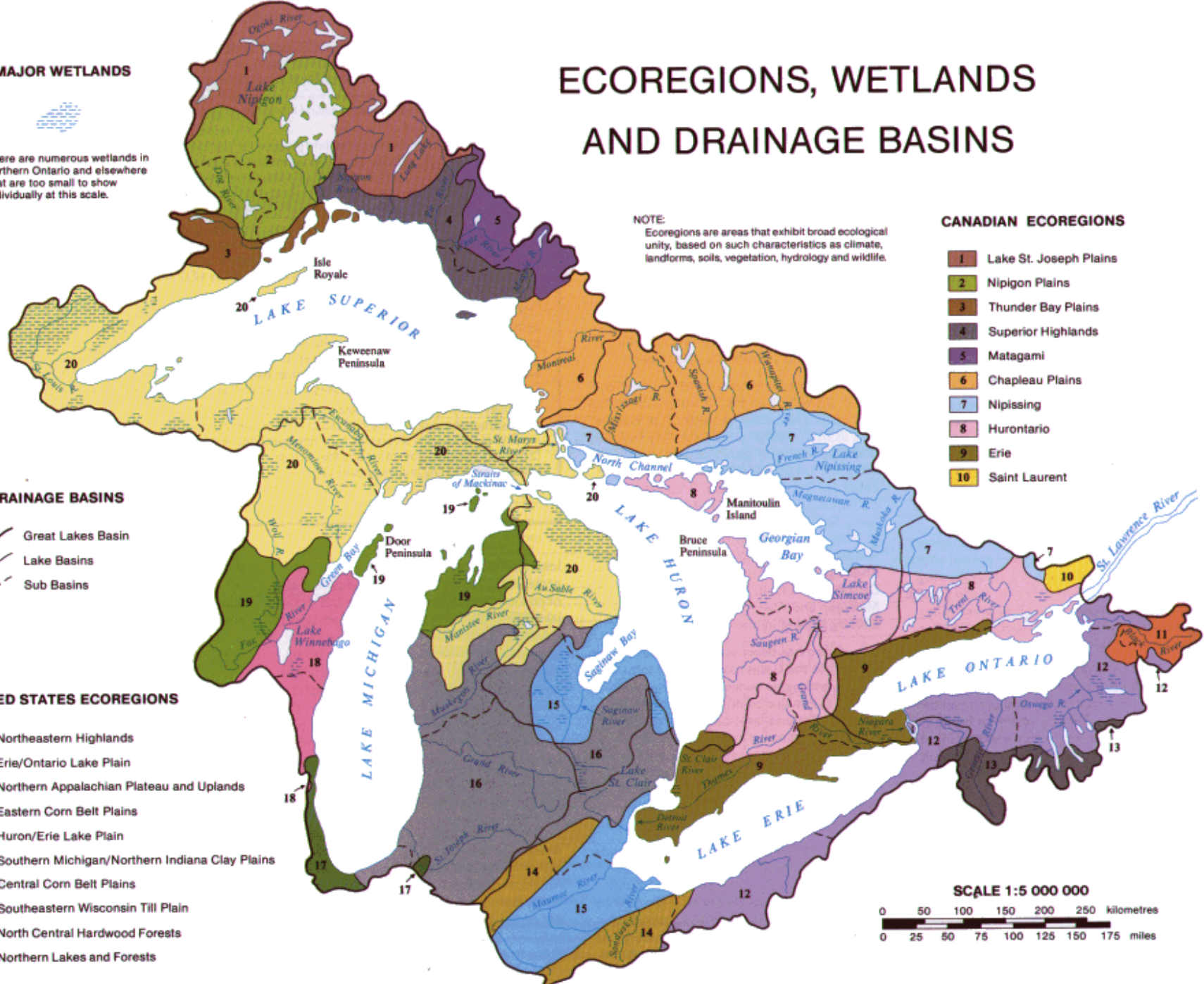
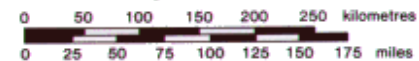
DRAINAGE BASINS

- Great Lakes Basin
- Lake Basins
- - - Sub Basins

UNITED STATES ECOREGIONS

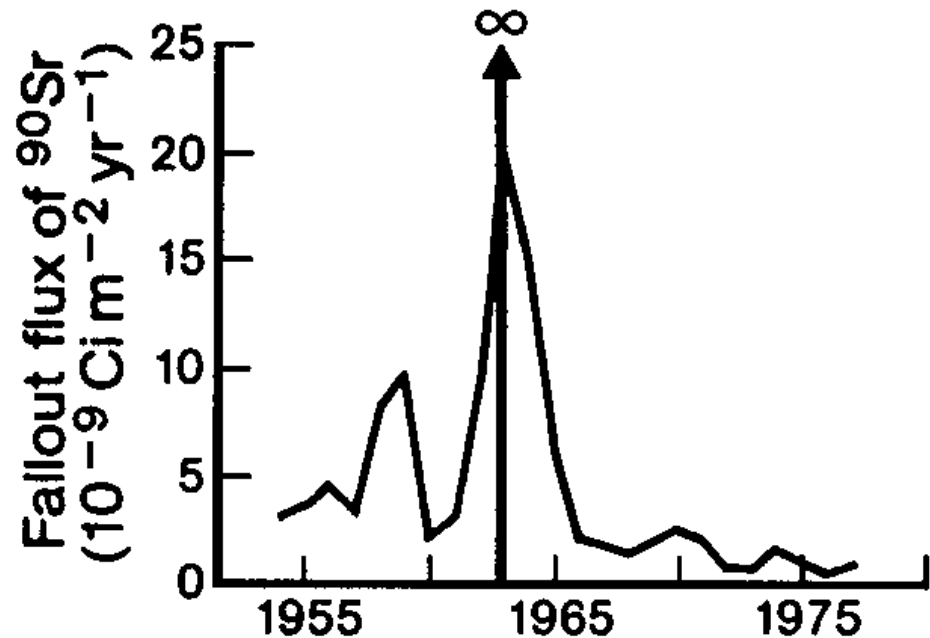
- 11 Northeastern Highlands
- 12 Erie/Ontario Lake Plain
- 13 Northern Appalachian Plateau and Uplands
- 14 Eastern Corn Belt Plains
- 15 Huron/Erie Lake Plain
- 16 Southern Michigan/Northern Indiana Clay Plains
- 17 Central Corn Belt Plains
- 18 Southeastern Wisconsin Till Plain
- 19 North Central Hardwood Forests
- 20 Northern Lakes and Forests

SCALE 1:5 000 000



Loading Function

- Close to an impulse load, centered around 1963
- estimated value is:
 $70 \times 10^{-9} \text{ Ci/m}^2$
- same for all lakes



$$\lambda = \frac{Q}{V} + k$$

Non Steady State Solution

Impulse Load

- 1st lake

$$c_1 = c_{10} e^{-\lambda_1 t}$$

 C_{11}

- 2nd lake

$$c_2 = c_{20} e^{-\lambda_2 t}$$

 C_{21}

$$+ c_{10} \frac{Q_{12}}{V_2(\lambda_2 - \lambda_1)} (e^{-\lambda_1 t} - e^{-\lambda_2 t})$$

 C_{22}

- 3rd lake

$$c_3 = c_{30} e^{-\lambda_3 t}$$

 C_{31}

$$+ c_{20} \frac{Q_{23}}{V_3(\lambda_3 - \lambda_2)} (e^{-\lambda_2 t} - e^{-\lambda_3 t})$$

 C_{32}

$$+ c_{10} \frac{Q_{23} Q_{12}}{V_3 V_2 (\lambda_2 - \lambda_1)} \left(\frac{e^{-\lambda_1 t} - e^{-\lambda_3 t}}{\lambda_3 - \lambda_1} - \frac{e^{-\lambda_2 t} - e^{-\lambda_3 t}}{\lambda_3 - \lambda_2} \right)$$

 C_{33}

Hydrologic Parameters

Parameter	Units	Superior	Michigan	Huron	Erie	Ontario
Mean Depth	m	146	85	59	19	86
Surface Area	10^6 m^2	82,100	57,750	59,750	25,212	18,960
Volume	10^9 m^3	12,000	4,900	3,500	468	1,634
Outflow	$10^9 \text{ m}^3/\text{yr}$	67	36	161	182	212

■ Michigan

$$c_m = c_{11}^m$$

■ Superior

$$c_s = c_{11}^s$$

■ Huron

$$c_h = c_{21}^h + c_{22}^{sh} + c_{22}^{mh}$$

■ Erie

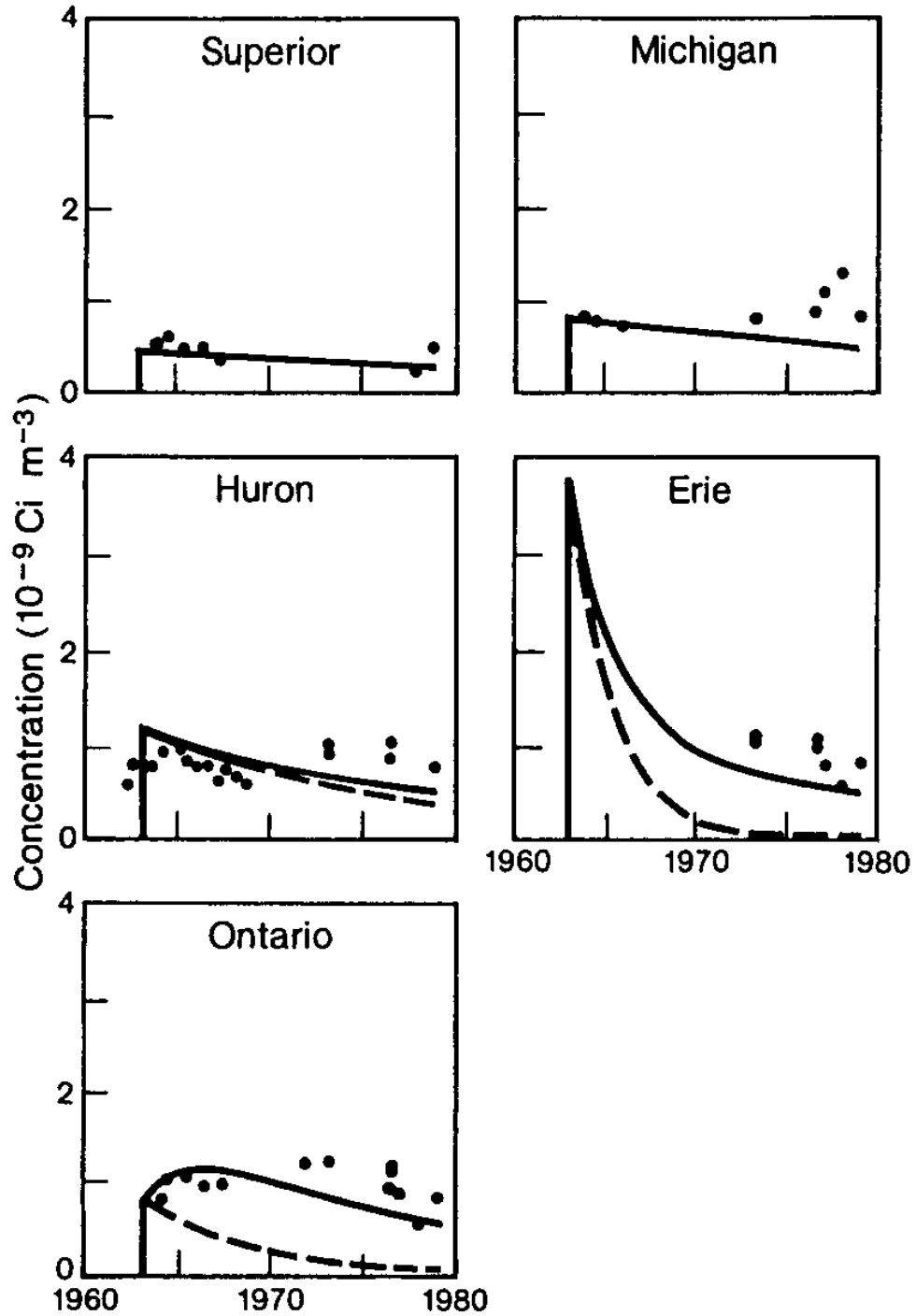
$$c_e = c_{31}^e + c_{32}^{he} + c_{33}^{she} + c_{33}^{mhe}$$

■ Ontario

$$c_o = c_{41}^o + c_{42}^{eo} + c_{43}^{heo} + c_{44}^{sheo} + c_{44}^{mheo}$$

$$\lambda = \frac{Q}{V} + k$$

- 
- From Chapra, 1997



- 
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- To next lecture